

MODELS FOR TENSION STIFFENING AND DEFLECTIONS OF GFRP-RC

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- **Introduction**
- **Problem definition**
- **Methodology**
 - **Direct tension studies to quantify Tension stiffening effect**
- **Modelling tension behaviour GFRP-RC**
- **Modelling Deflections**
- **Conclusions and remarks**

Introduction: GFRP RC in Construction



Corrosions of Steel reinforcements



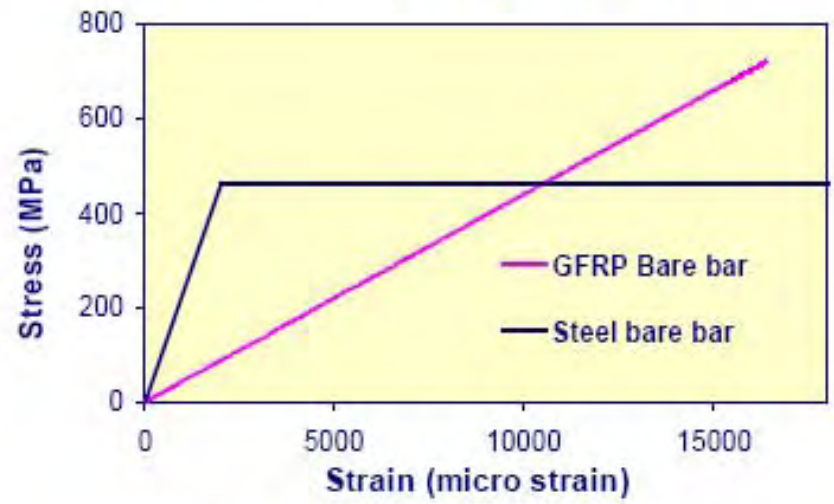
Use of GFRP for bridge deck construction (Franklin county bridge Virginia)



GFRP bars



Steel bars



Stiffness of GFRP compared to Steel

Tension Stiffening in Design Codes

In ACI

$$\Delta = \frac{kPl^3}{EI_{eff}}$$

Branson's equation for I_{eff}

$$I_{eff} = I_g \left(\frac{M_{cr}}{M_a} \right)^3 + I_{cr} \left[1 - \left(\frac{M_{cr}}{M_a} \right)^3 \right]$$

ACI 440 approach

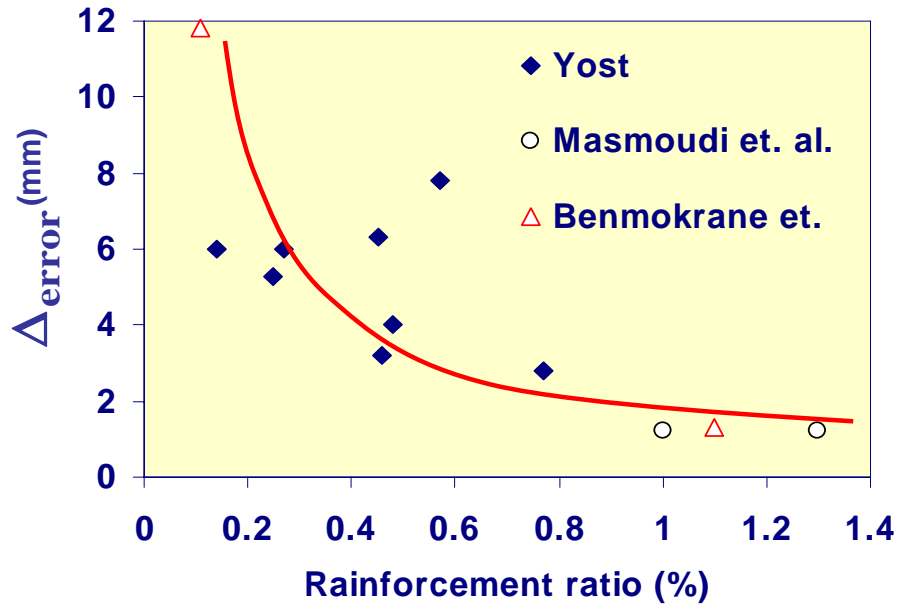
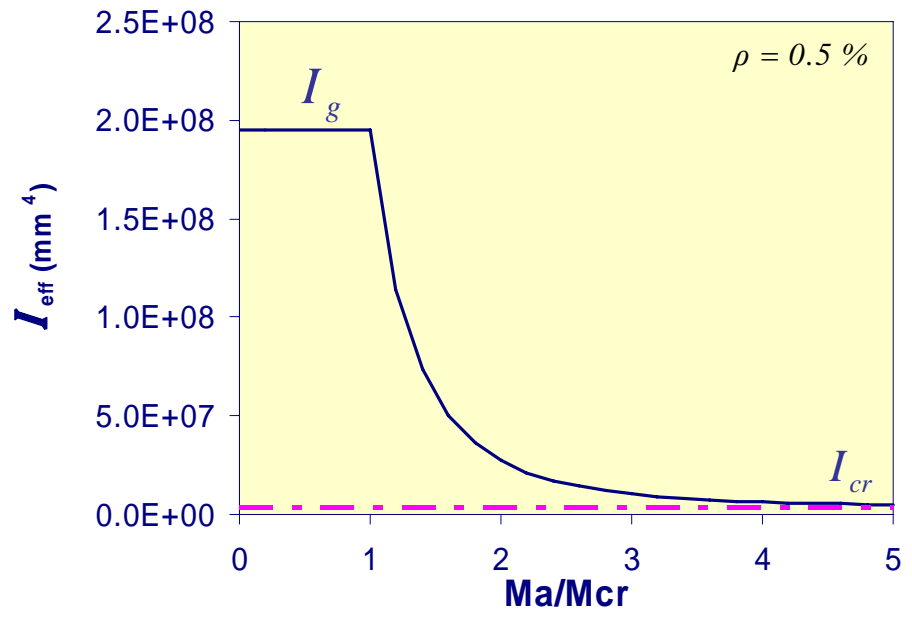
$$I_{eff} = I_g \beta_d \left(\frac{M_{cr}}{M_a} \right)^3 + I_{cr} \left[1 - \left(\frac{M_{cr}}{M_a} \right)^3 \right]$$

$$\beta_d = \alpha_b \left[\frac{E_f}{E_s} + 1 \right]$$

$$\alpha_b = 0.5$$

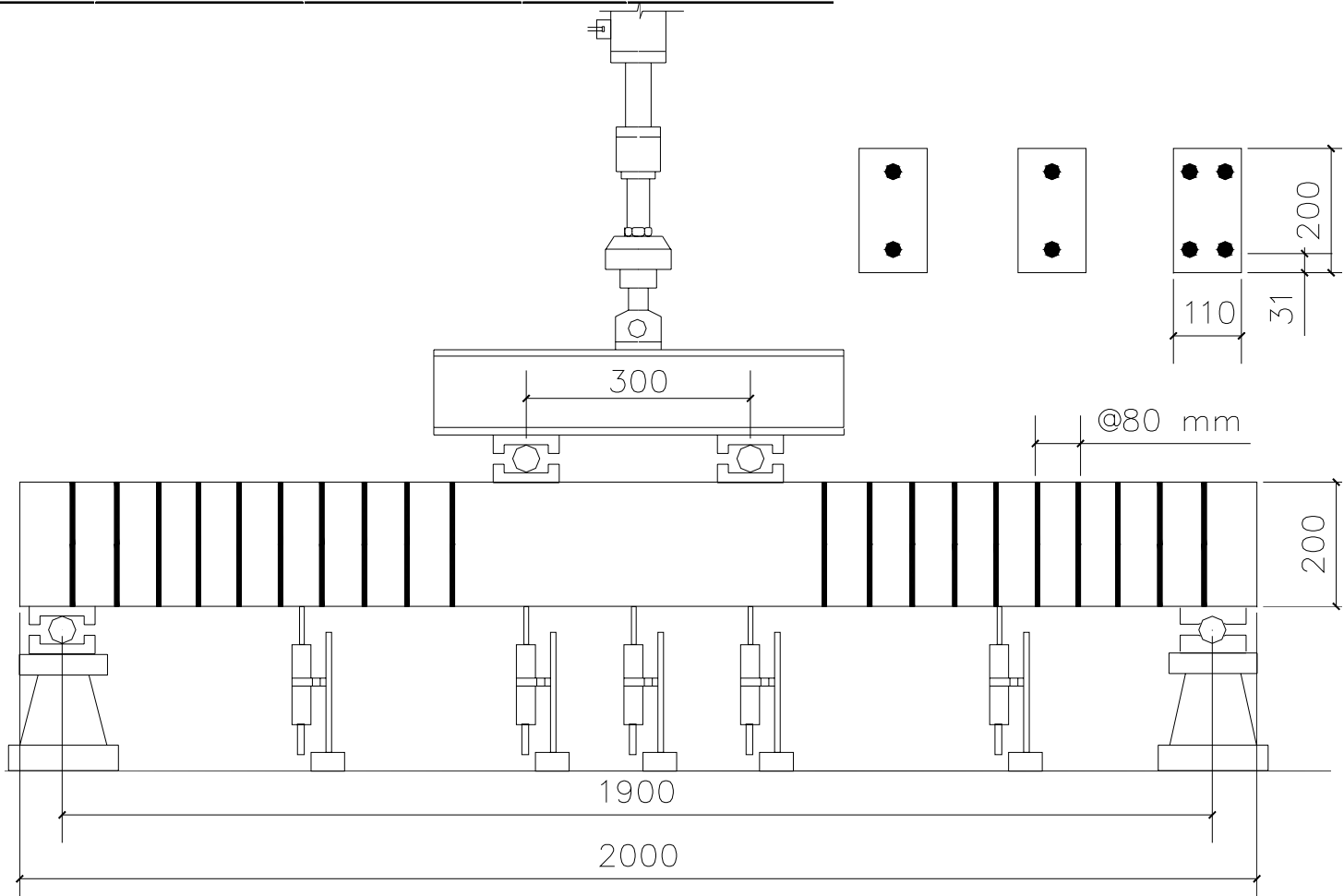
Δ_{error} = Experimental Deflection minus the Deflection by Branson's Equation, both at service level (50% ultimate load)

(courtesy Toutanji et al. (2003), Construction and Building Material)



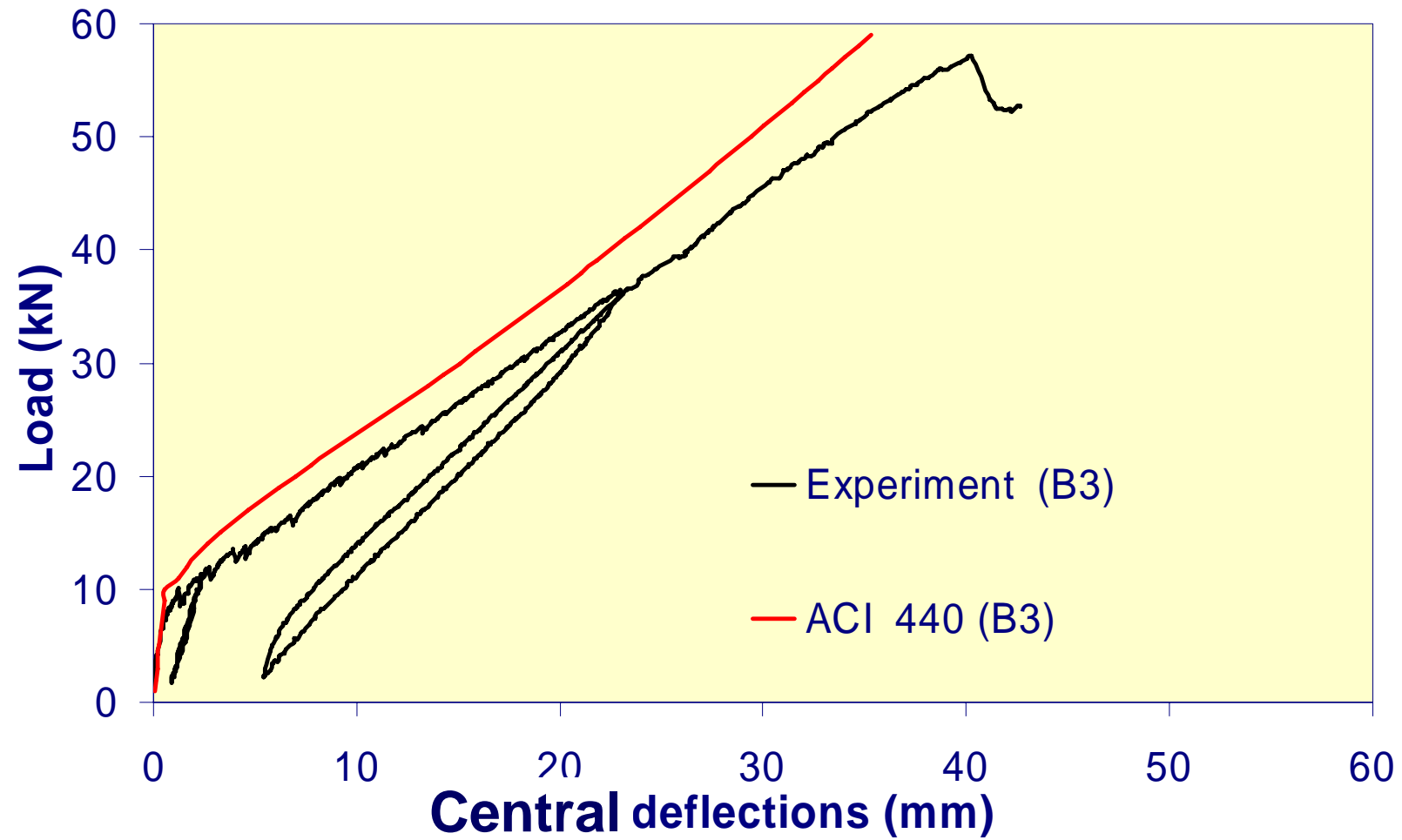
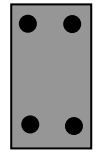
Test on deflections –beam series

Rebar type	Beam	Cover (mm)	Reinforcement	Rein. Ratio $\rho = A_s / bh$ (%)	f_{cu} (MPa)	Failure mode
	B1	31	1Ø12.7 mm	0.57	91	Bar failure
GFRP	B2	31	1Ø12.7 mm	0.57	46	Con. Crushing
	B3	31	2Ø12.7 mm	1.15	Con. Crushing	



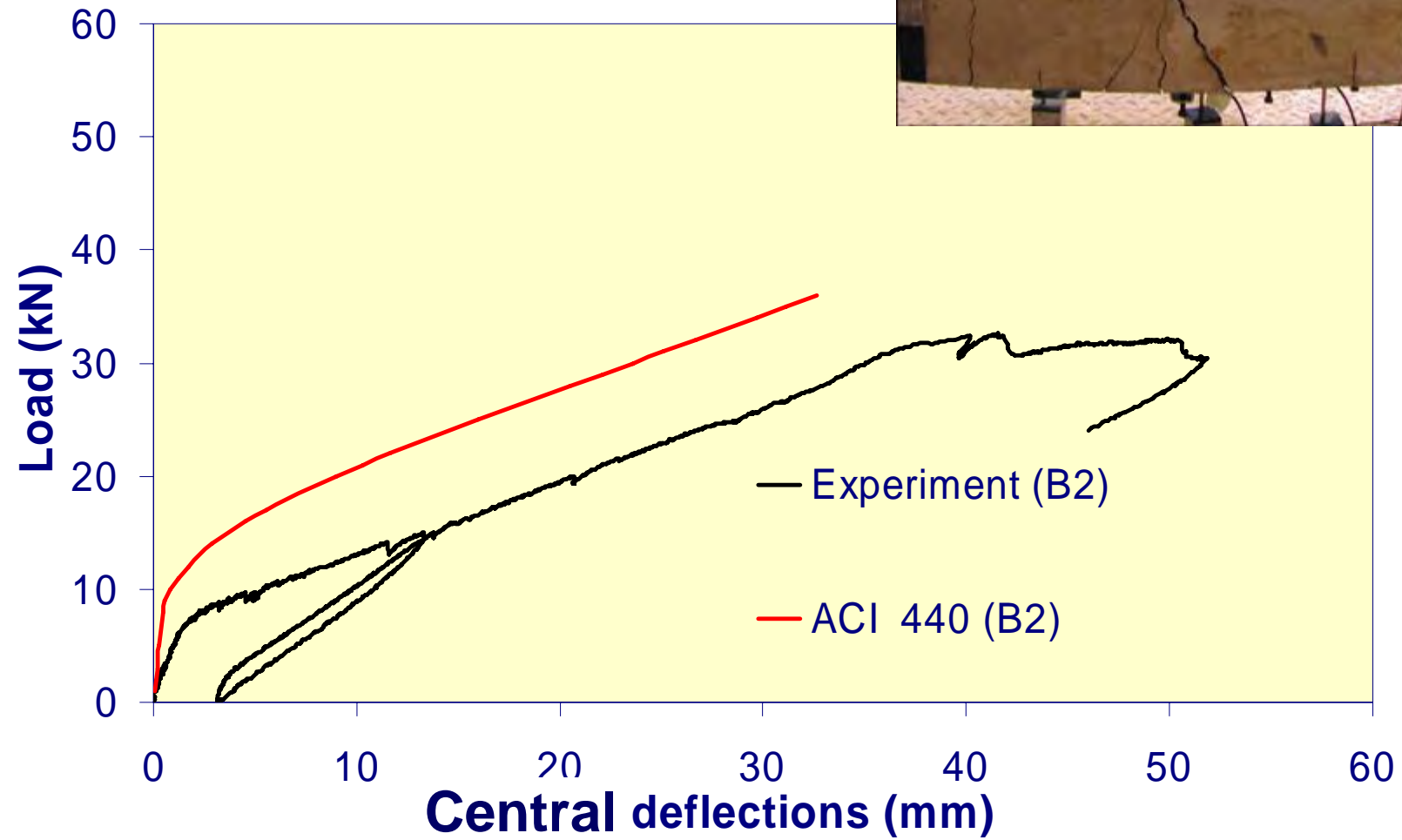
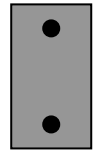
Tests on deflections –beam series (B3)

$\rho = 1.15$



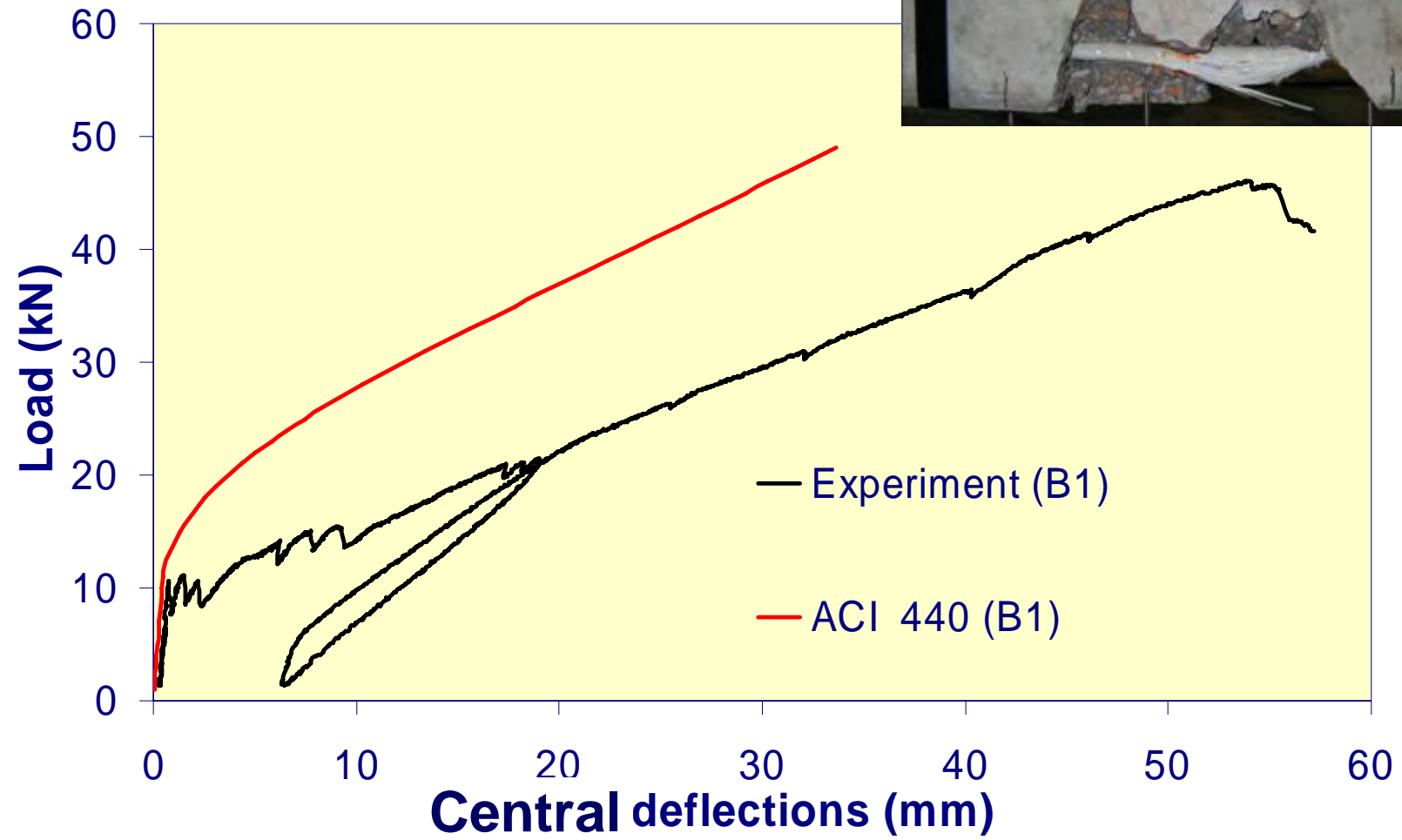
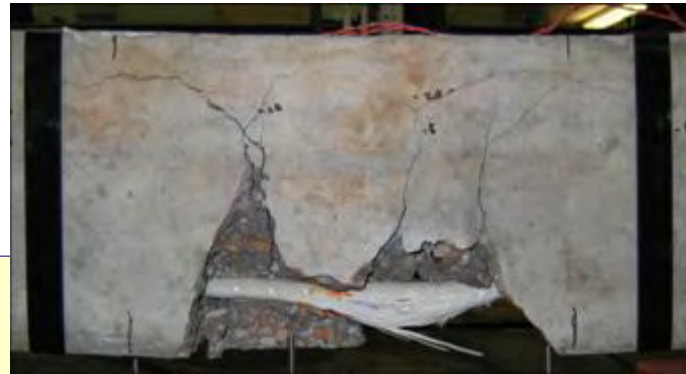
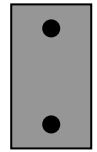
Tests on deflections –beam series (B2)

$$\rho = 0.57$$



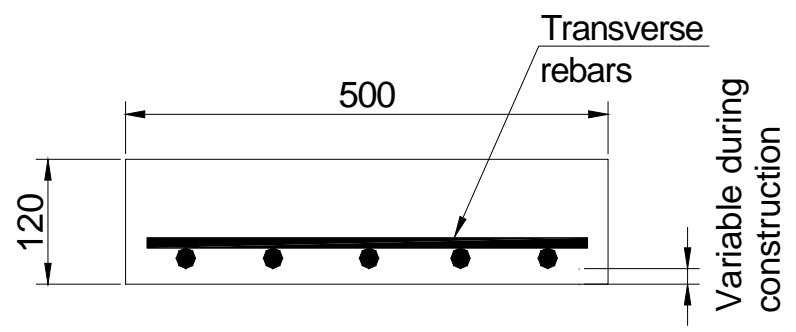
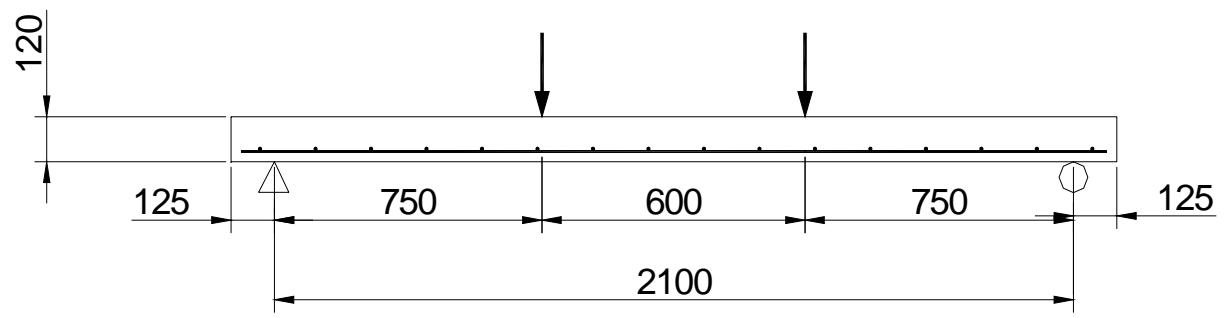
Tests on deflections –beam series (B1)

$$\rho = 0.57$$



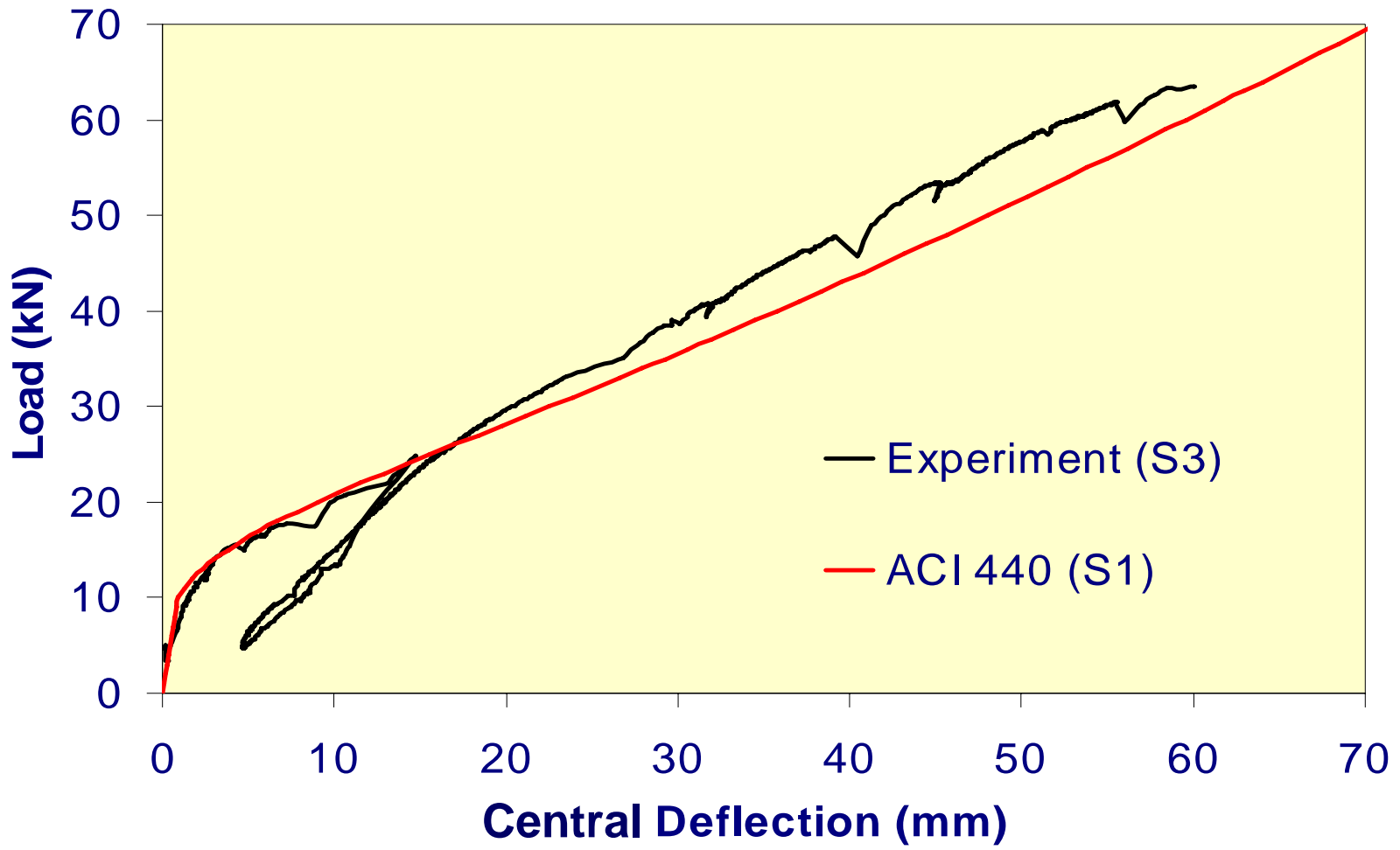
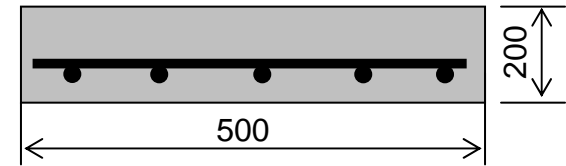
Tests on deflections –Slab series

Rebar type	Slab	Cover, (mm)	Reinforcement	Reinforcement Ratio $\rho = A_s / bh$ (%)	Concrete Cylinder strength “MPa”
GFRP	S1	27.5	5 \varnothing 6mm	0.24	43
	S2	31	5 \varnothing 9.53mm	0.59	39
	S3	40	5 \varnothing 19.05mm	2.38	39



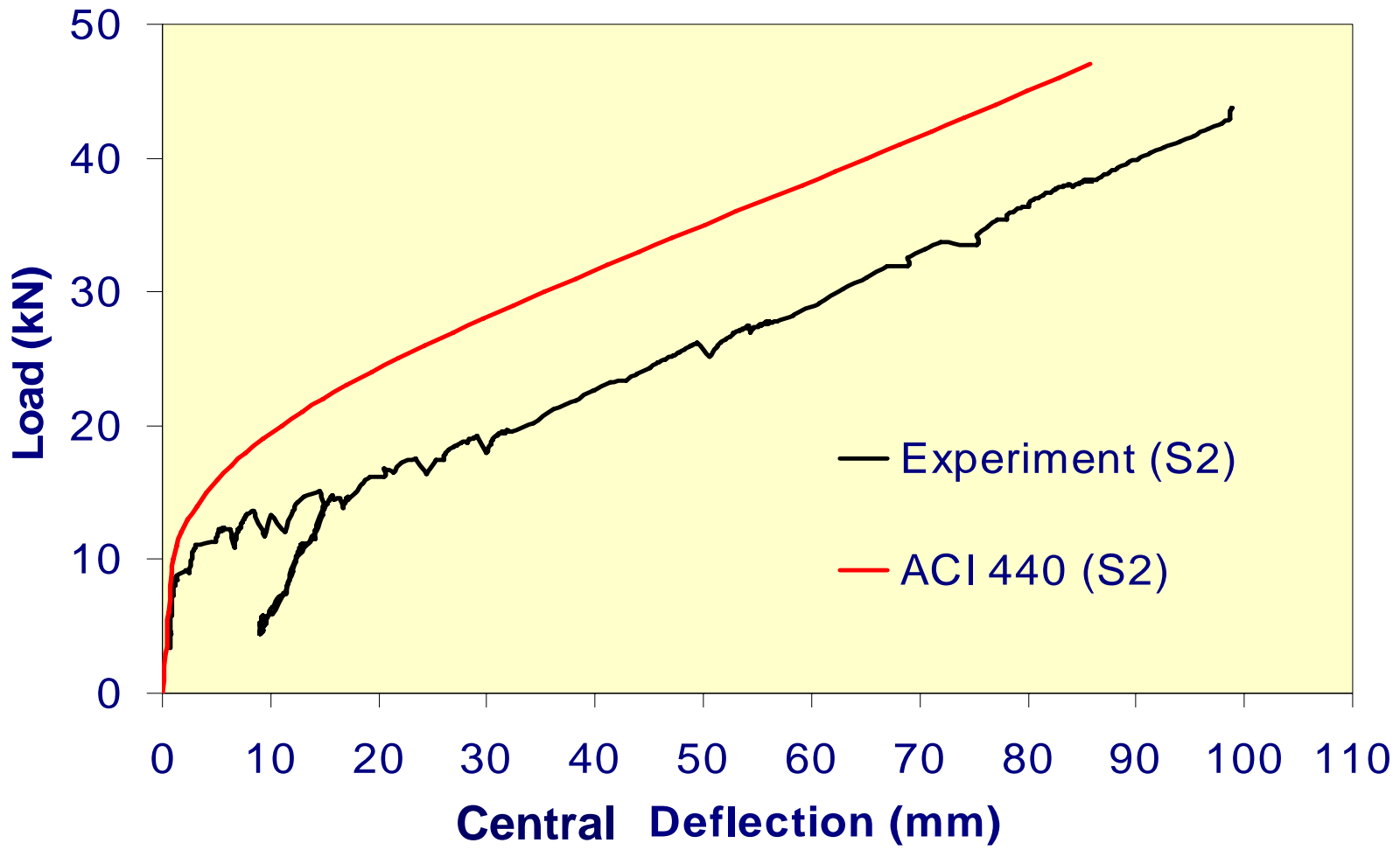
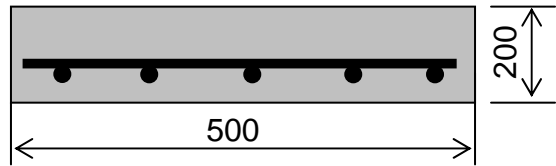
Test on deflections –beam series (S3)

$$\rho = 2.38$$



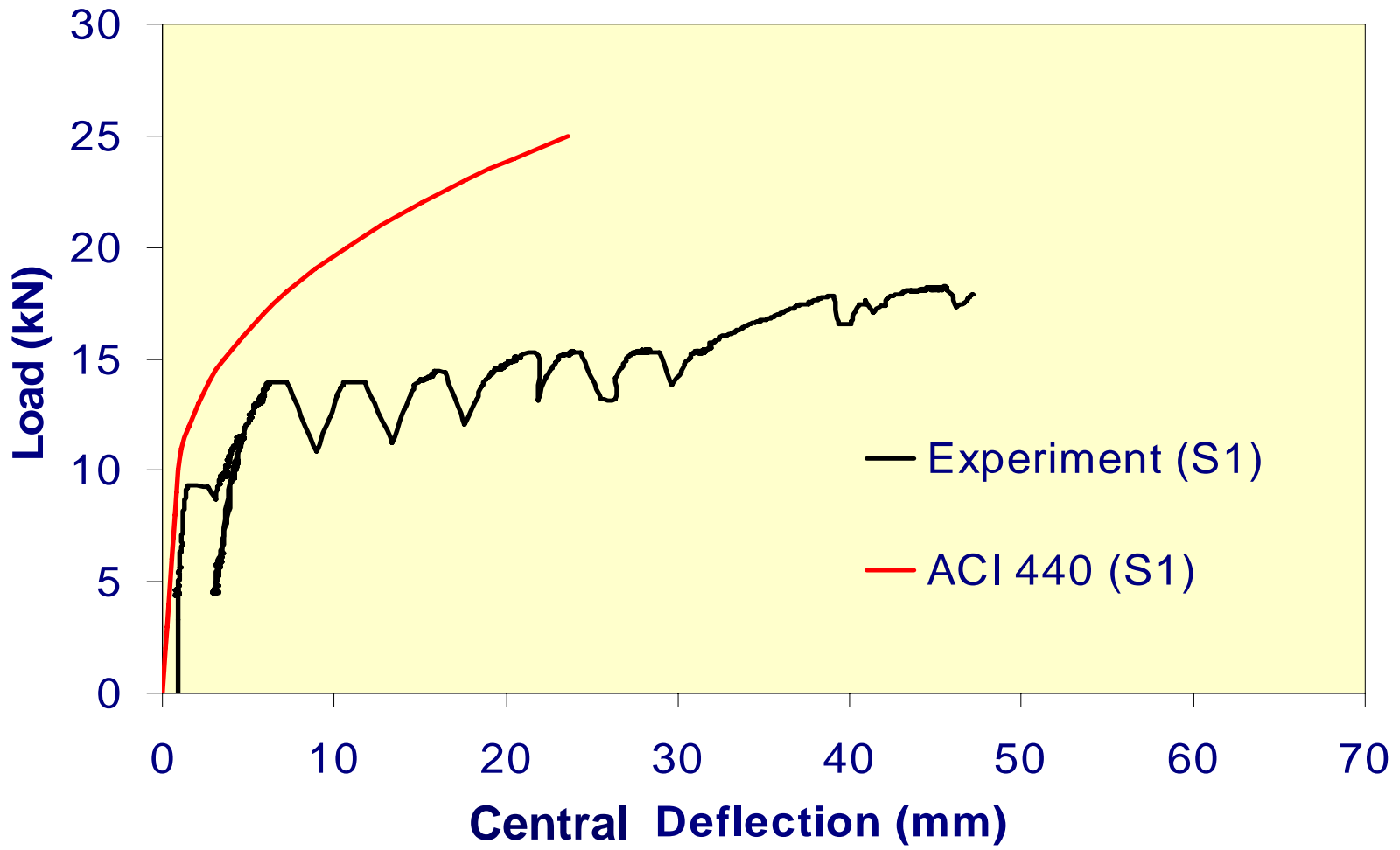
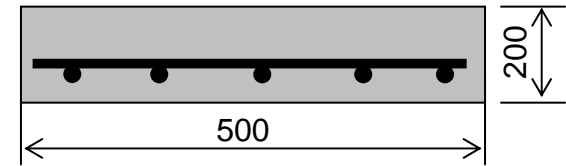
Test on deflections –beam series (S2)

$$\rho = 0.59$$



Test on deflections – beam series (S1)

$$\rho = 0.24$$



- No general agreement on tension stiffening

$$I_{eff} = I_g \left(\frac{M_{cr}}{M_a} \right)^3 + I_{cr} \left[1 - \left(\frac{M_{cr}}{M_a} \right)^3 \right] \quad \text{ACI Branson's}$$

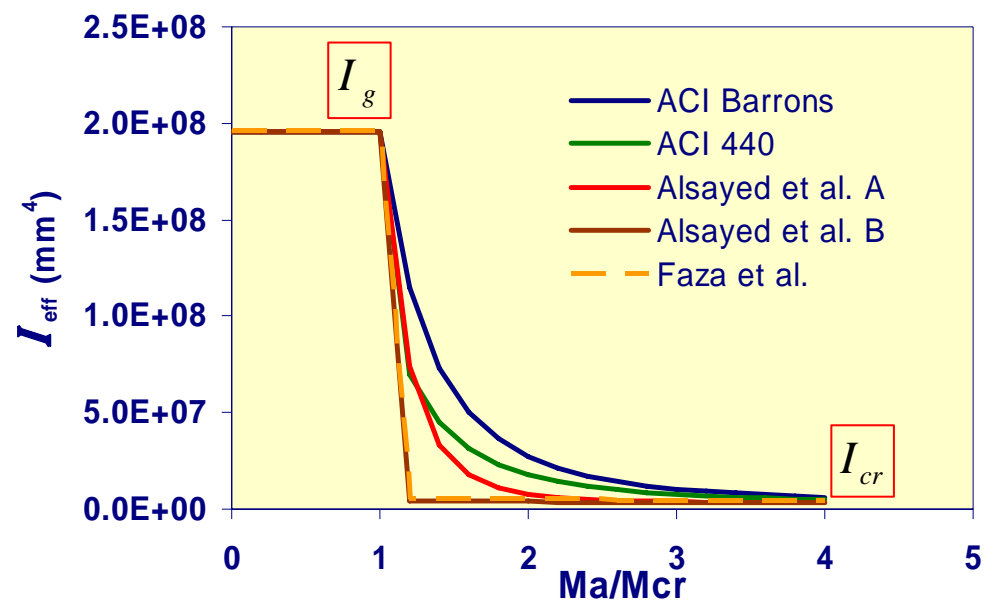
$$I_{eff} = I_g \beta_d \left(\frac{M_{cr}}{M_a} \right)^3 + I_{cr} \left[1 - \left(\frac{M_{cr}}{M_a} \right)^3 \right] \quad \text{ACI 440}$$

$$I_m = \frac{23I_{cr}I_e}{8I_{cr} + 15I_e}, \quad I_e = \text{ACI} \quad I_{eff} \quad \text{Faza et. al B}$$

$$I_{eff} = I_g \left(\frac{M_{cr}}{M_a} \right)^{5.5} + I_{cr} \left[1 - \left(\frac{M_{cr}}{M_a} \right)^{5.5} \right] \quad \text{Alsayed et. al A}$$

$$1 < \frac{M_a}{M_{cr}} < 3 \Rightarrow I_{eff} = I_{cr} \left[1.40 - \frac{2}{15} \left(\frac{M_a}{M_{cr}} \right) \right] \quad \text{Alsayed et. al B}$$

$I_{eff} = I_{cr}$



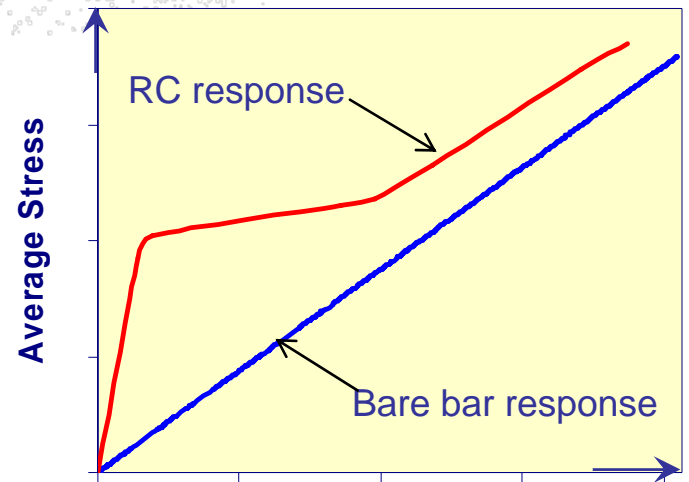
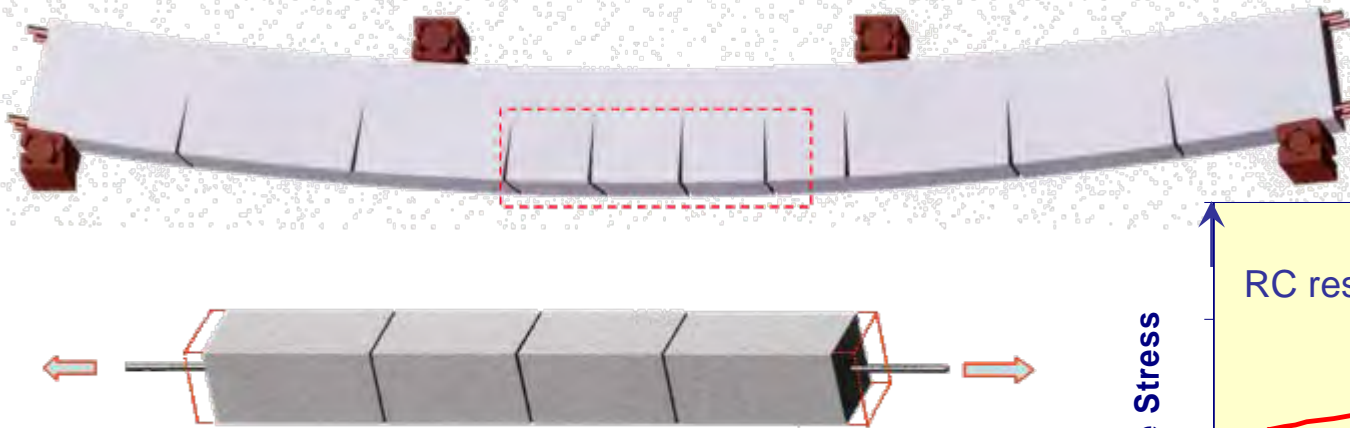
- **Study the tension stiffening effect at fundamental level**
- **Develop a suitable way to incorporate tension stiffening in deflection**

Studying tension stiffening alone was important at the time as it is necessary for Modelling GFRP-RC using FE Method based on smeared crack approach.

Definition- Tension Stiffening

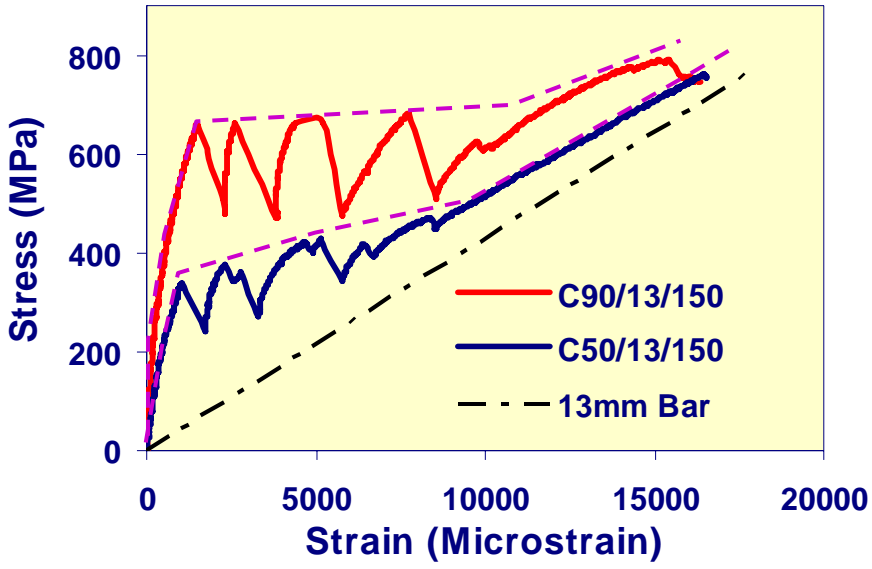
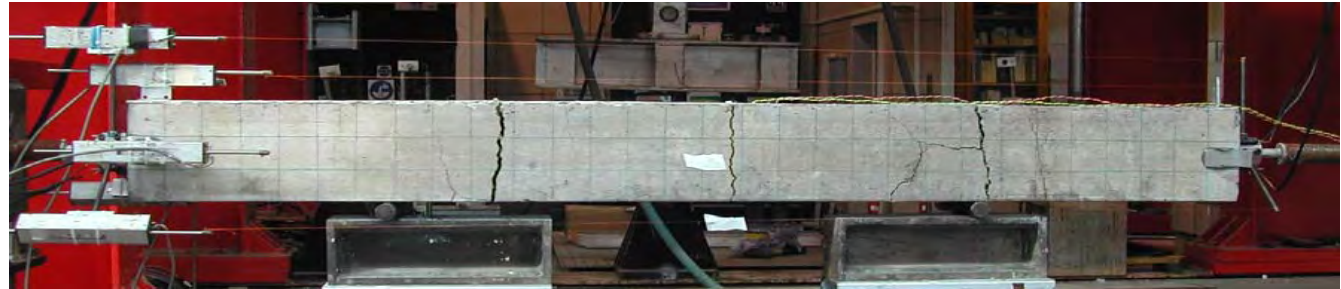
Tension stiffening of concrete is defined as:
the ability of concrete to carry tension between cracks and provide extra stiffness for RC in tension.

- **Serviceability** often governs GFRP-RC design
- **Tension stiffening is very important for the determination of deflections and crack widths at low load levels**

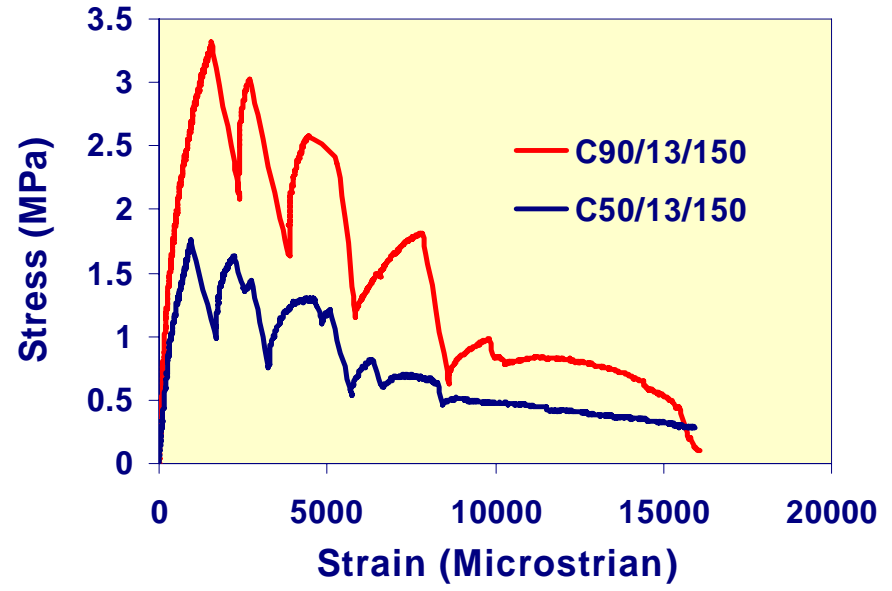


Tension stiffening effect

Strain Softening Behaviour Concrete

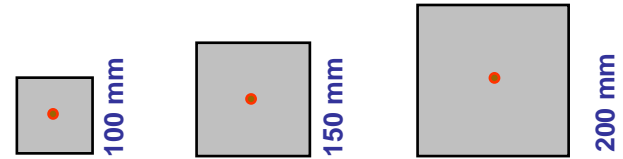


Test results bar stress Vs overall strain



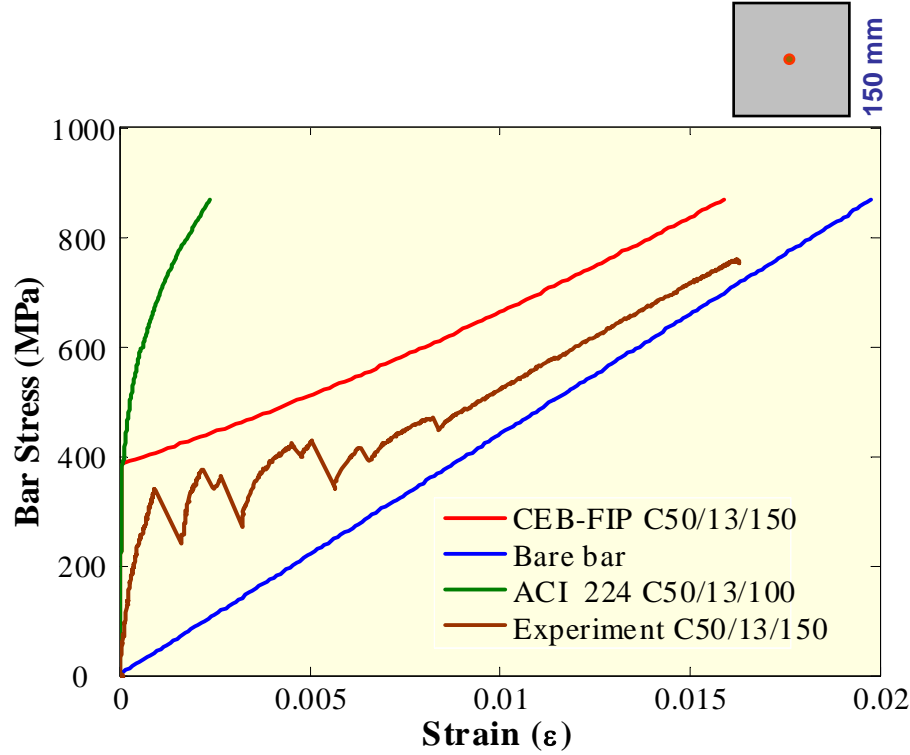
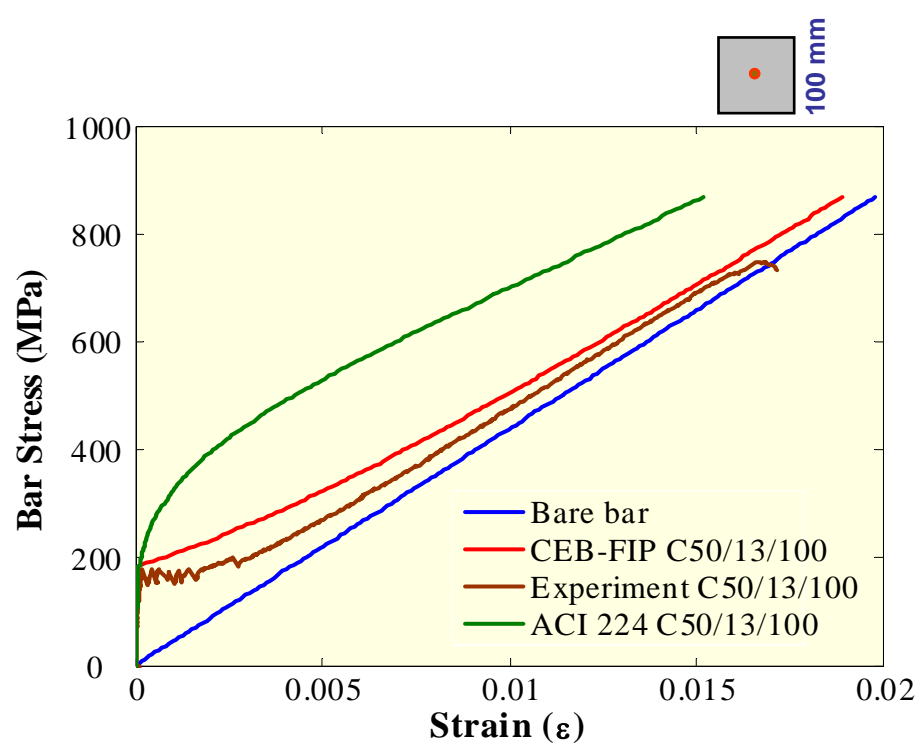
Average stress strain behaviour of concrete

Parametric Study



Specimen	Concrete strength (f_c')	Bar Diameter (ϕ)	Dimension $b \times d \times l$	Reinforcement ratio (ρ)
C50/13/100	46	12.7	100×100×1500	1.26
C50/13/150	46	12.7	150×150×1500	0.56
C50/13/200	46	12.7	200×200×1500	0.32
C90/13/100	91	12.7	100×100×1500	1.26
C90/13/150	91	12.7	150×150×1500	0.56
C50/19/150	46	19.1	150×150×1500	1.27
C50/19/200	46	19.1	200×200×1300	0.72
C90/19/150	46	19.1	150×150×1300	1.27
C90/19/200	91	19.1	200×200×1300	0.72
C50/19/200N	91	19.1	200×200×1300	0.72

Prediction of tension stiffening effect - Code based approach



Reduced cross sectional area

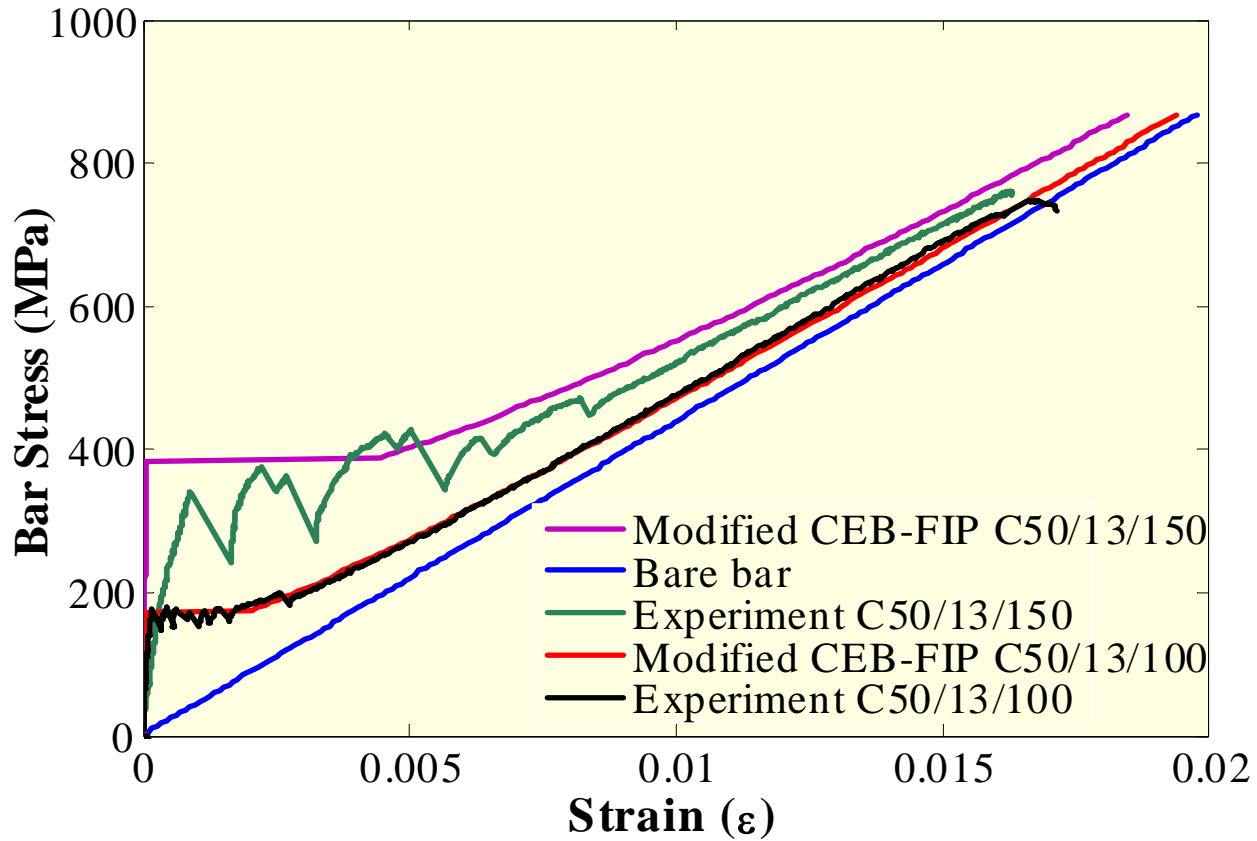
Original
$$A_e = \left[\frac{P_{cr}}{P_a} \right]^3 A_g + \left[1 - \left(\frac{P_{cr}}{P_a} \right)^3 \right] A_{cr}$$
 ACI

Modified to account for weak FRP bond
$$A_e = \left[\frac{P_{cr}}{P_a} \right]^3 \beta_d A_g + \left[1 - \left(\frac{P_{cr}}{P_a} \right)^3 \right] A_{cr}$$

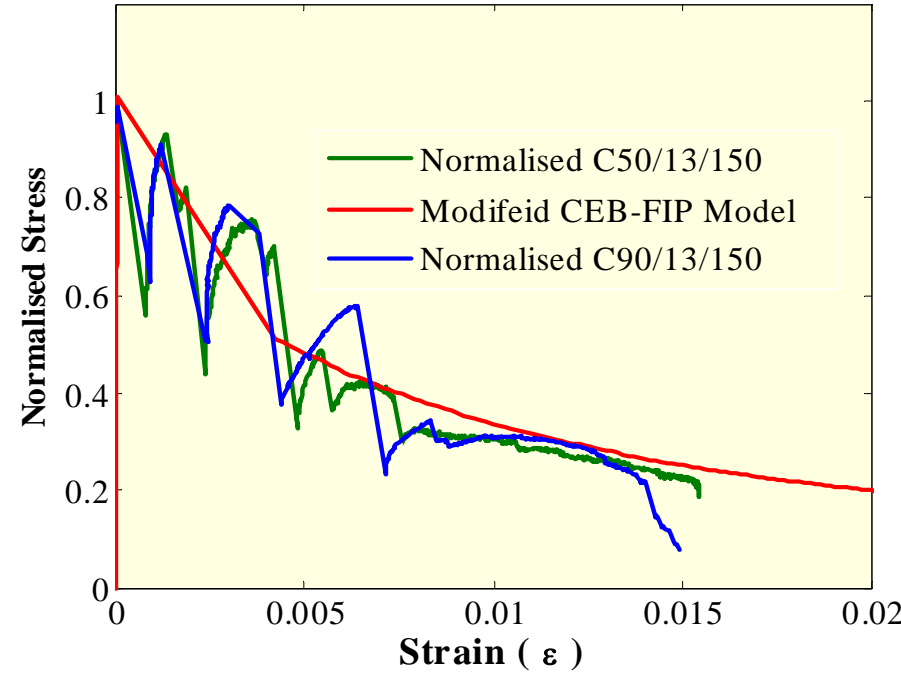
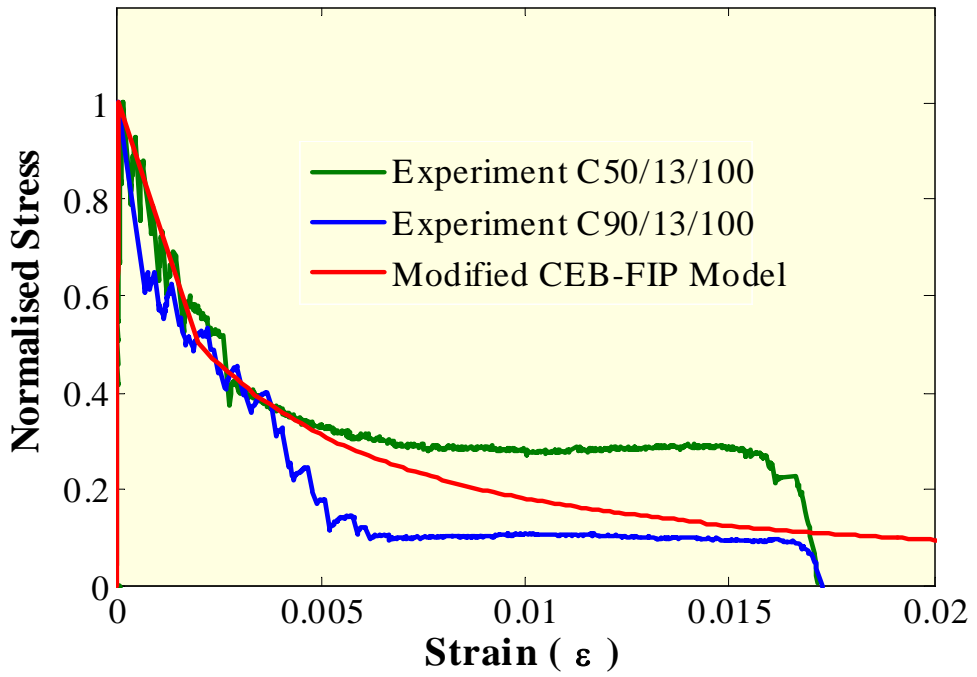
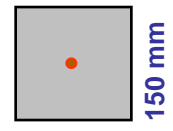
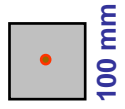
Composite strain for given bar strain

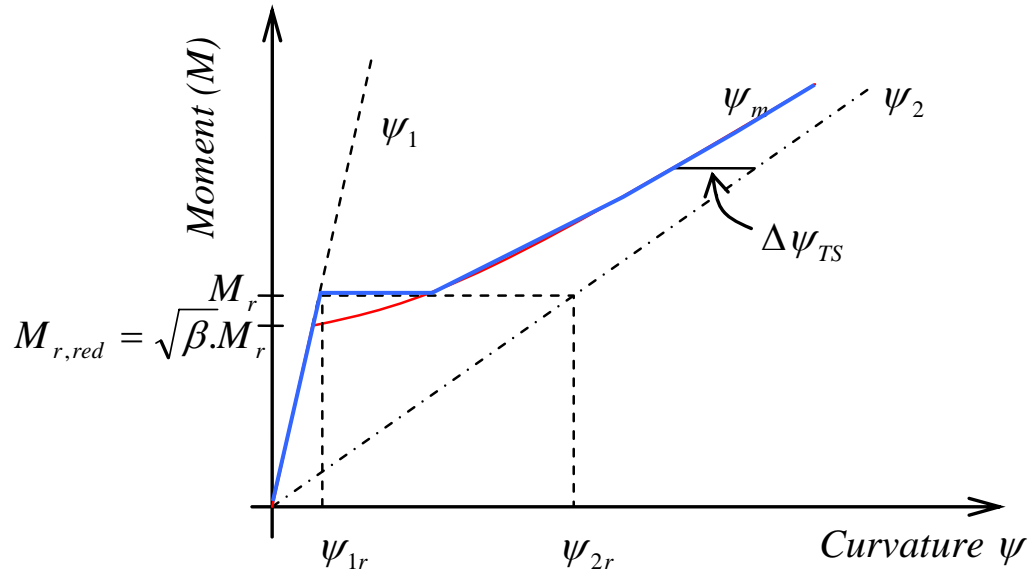
$$\epsilon_m = \epsilon_s \left[1 - K \left(\frac{f_{scr}}{f_f} \right)^2 \right]$$
 CEB

$$f_{scr} = \frac{P_{cr}}{A_f} = f_t' \left(\frac{1}{\rho} - 1 + n_f \right)$$



$$\epsilon_m = \epsilon_s \left[1 - 0.5 \left(\frac{f_{scr}}{f_f} \right)^2 \right]$$





Moment curvature relationship

$$\psi_m = \psi_1$$

$$\psi_m = \psi_2 - \Delta\psi_{TS}$$

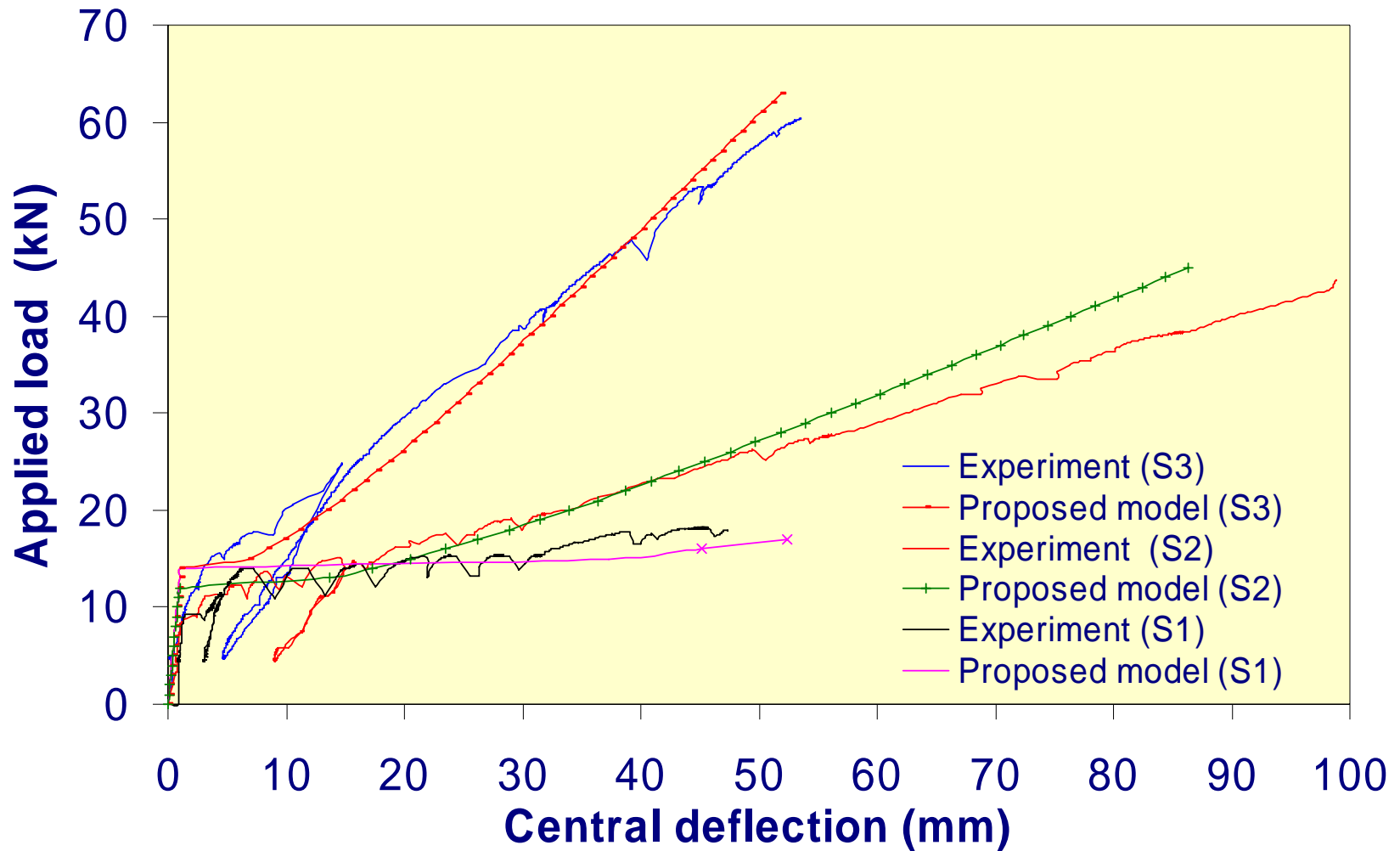
$$\Delta\psi_{TS} = (\psi_{2r} - \psi_{1r})\beta \frac{M_r}{M}$$

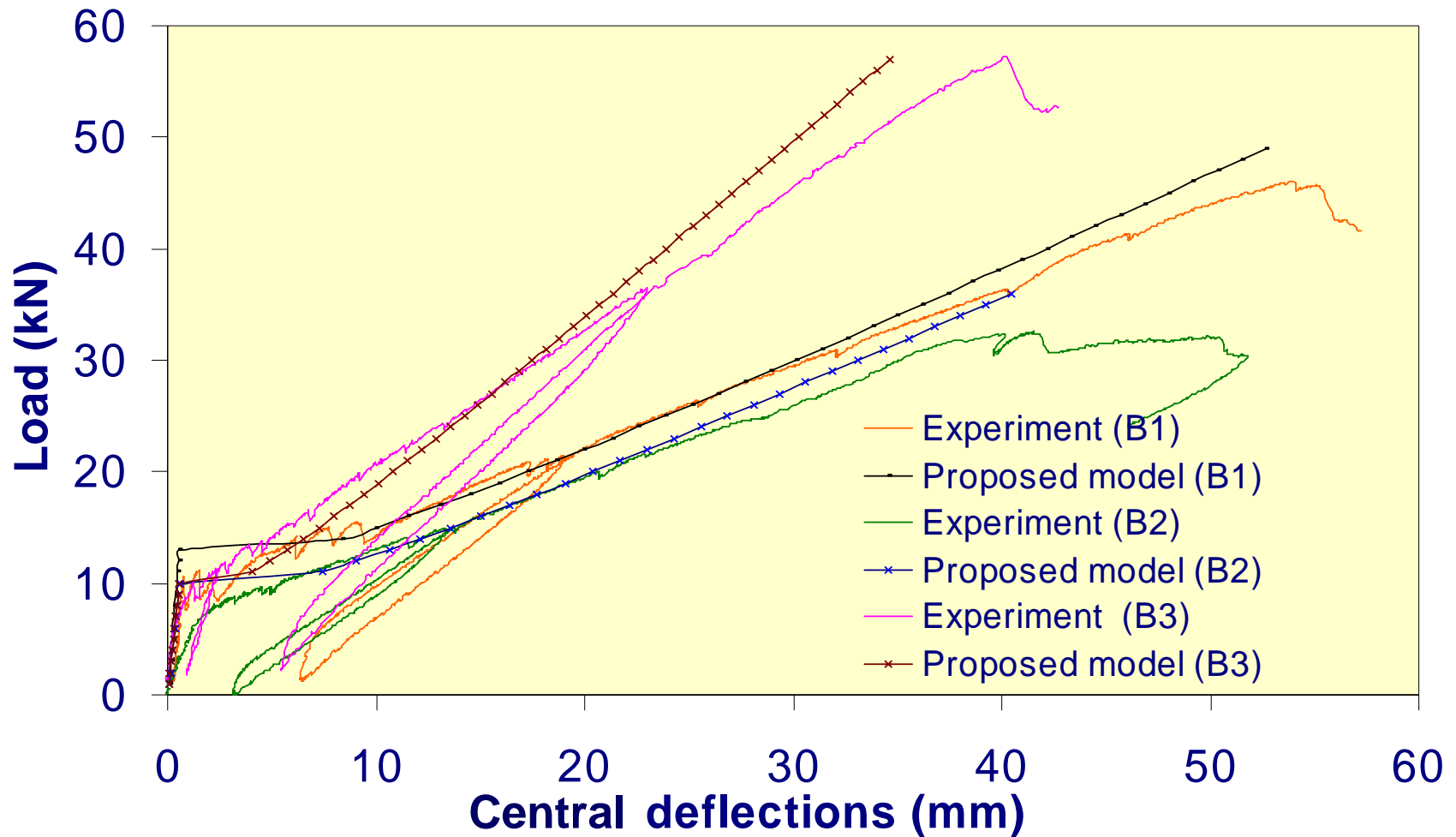
$$\psi_m = \psi_2 - (\psi_2 - \psi_1)\beta \left(\frac{M_r}{M}\right)^2$$

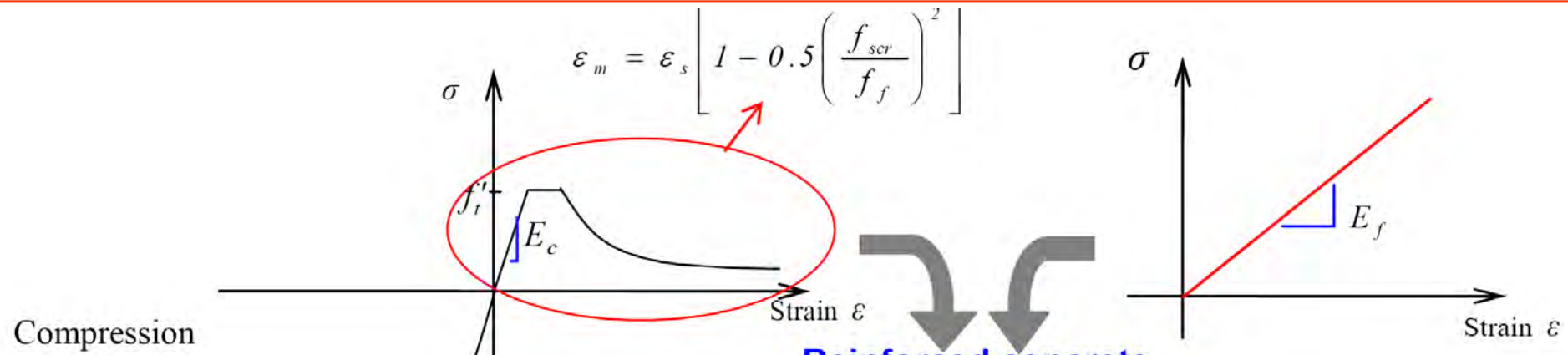
Simplified deflection relationship

$$a = a_2 - (a_1 - a_2)\beta \left(\frac{M_{rD}}{M_D}\right)^2$$

$$a = a_2 - (a_1 - a_2)0.5 \left(\frac{M_{rD}}{M_D}\right)^2$$



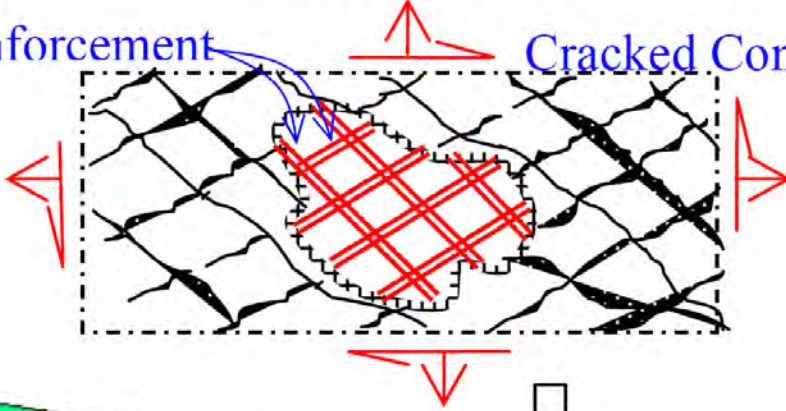




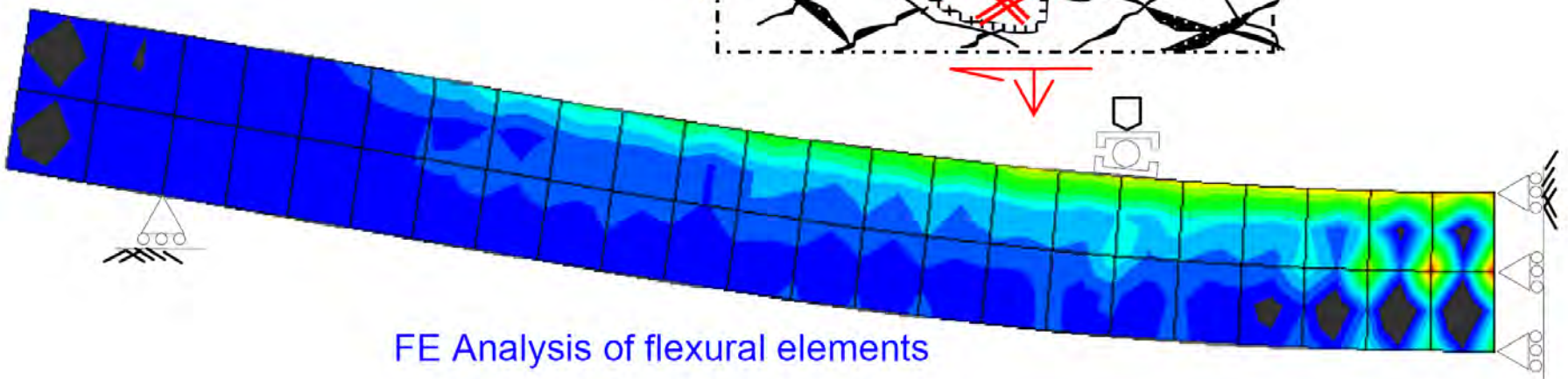
$$\epsilon_m = \epsilon_s \left[1 - 0.5 \left(\frac{f_{scr}}{f_f} \right)^2 \right]$$

Reinforced concrete

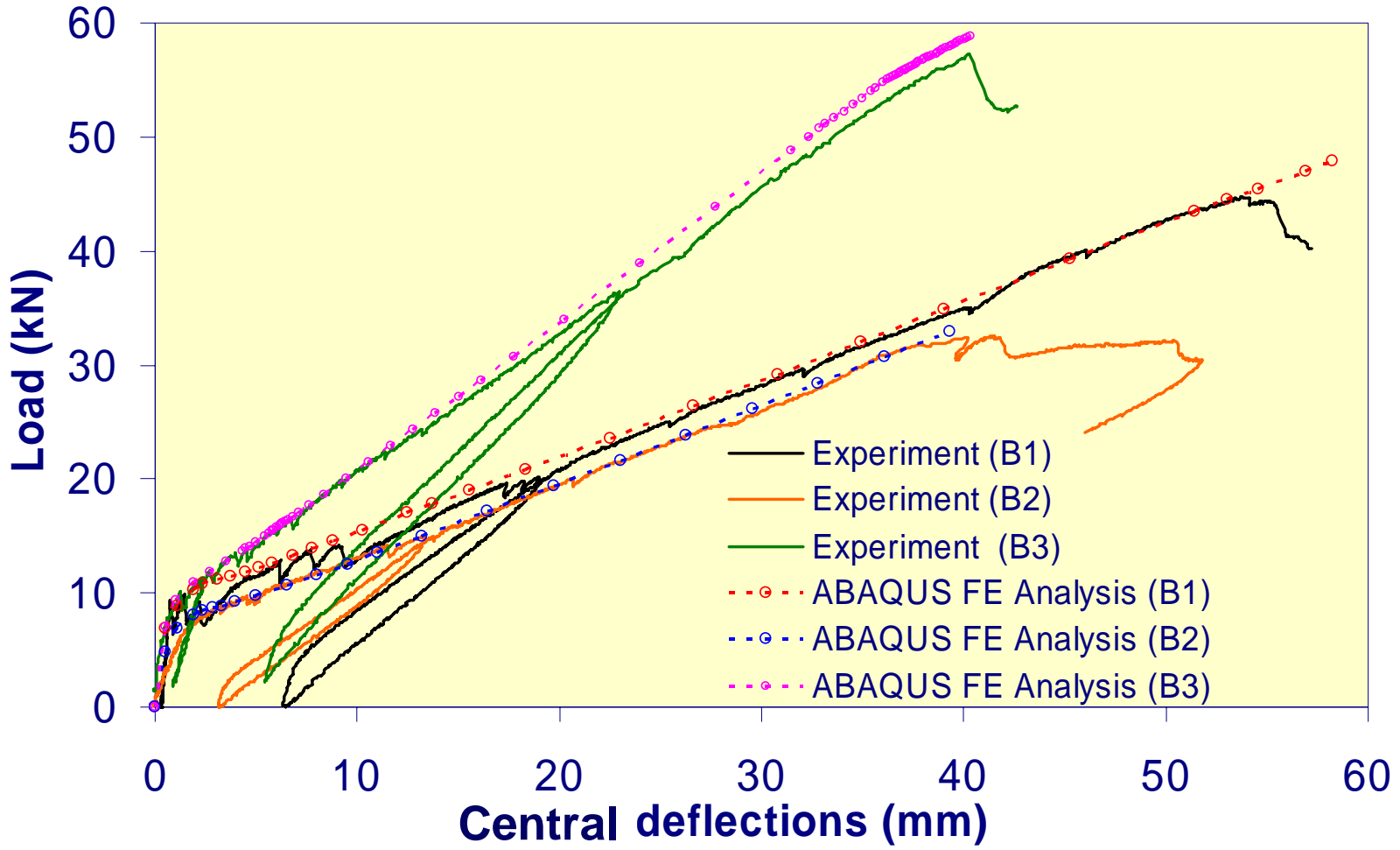
Reinforcement Cracked Concrete

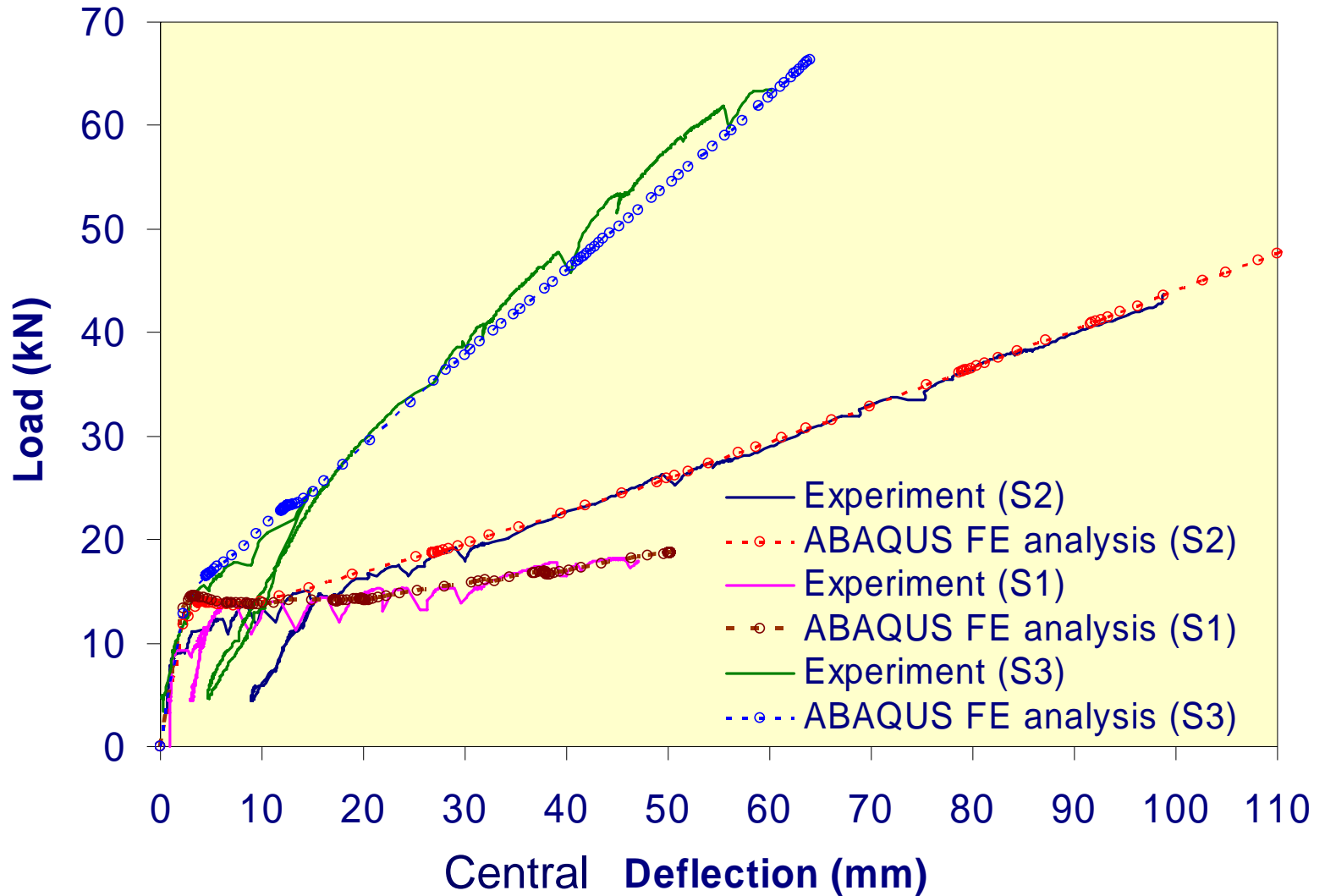


There are many models to explain the compression behaviour
BS8110 Part II, CEB-FIP



FE Analysis of flexural elements





- Direct tension test is used to study the tension stiffening effect
- Existing ACI equation are not suitable for predicting tension stiffening effect or deflections
- Study proposes accurate model to account for tension stiffening effect

$$\varepsilon_{cf} = \varepsilon_f \left[1 - 0.5 \left(\frac{f_{scr}}{f_f} \right)^2 \right]$$

- With the proposed tension stiffening model accurate consistent deflection predictions is possible.
- Simplified version of deflection predictions are also shown possible with the proposed equation

$$a = a_2 - (a_1 - a_2) 0.5 \left(\frac{M_{rD}}{M_D} \right)^2$$